# Estimation of the Fundamental Frequency of Beams and Plates with Varying Thickness

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## I. Introduction

perturbation method presented here provides an approximate technique for the solution of the free vibration of beams and plates with some general variation in thickness. The advantage of the perturbation method over the other approximate techniques, such as Rayleigh-Ritz, Galerkin, or finite element, is that once the solution is determined for a general variation of thickness then the effect of small changes in thickness variation can be determined without resolving the original free-vibration problem. The simplicity and speed of the perturbation method should make it a valuable tool to the aerospace structural designer for determining the vibrational characteristics of least-weight beams and plates. Chehil and Dua<sup>1</sup> have previously used a perturbation method to determine the buckling stress of plates with variable thickness.

#### II. Development of Perturbation Equations for Free Motion

#### A. Free Vibration of Beams

The differential equation describing free transverse motion of a beam (with variable thickness and unit width) is

$$d^{2}/dx[(Eh^{3}/12)(d^{2}w/dx^{2})] - \rho h\omega^{2}w = 0$$
 (1)

where w is the modal function, E is Young's modulus,  $\rho$  is mass density,  $\omega$  is the frequency of oscillation, and h is the beam thickness. It is assumed that the height of the nonuniform beam shown in Fig. 1 varies only slightly from some reference uniform beam of height  $h_{\theta}$ , so that

$$h(x) = h_0 + \epsilon h_1(x) \tag{2}$$

where  $\epsilon$  is a parameter here assumed to be less than unity, giving a measure of the variation from the reference uniform beam, and max  $h_I$  is equal to or less than  $h_0$ .

The fundamental frequency and mode shape of the beam with variable thickness not differing too greatly from the uniform reference beam are represented as 4

$$\omega^2 = \omega_0^2 + \sum_{i=1}^{I} \epsilon^i \omega_i^2 \tag{3}$$

and

$$w(x) = w_0(x) + \sum_{i=1}^{I} \epsilon^i w_i(x)$$
 (4)

where  $\omega_0^2$  and  $w_0(x)$  are the fundamental frequency squared and mode shape, respectively, of the reference beam with uniform thickness  $h_0$ . Equations (2-4) are substituted into Eq. (1), which, after neglecting terms of the order of  $\epsilon^3$  and higher

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and equating terms of equal powers of  $\epsilon$ , yields the following perturbation equations.

Zeroth-order Perturbation Equation

$$EI_0 d^4 w_0 / dx^4 - \rho h_0 \omega_0^2 w_0 = 0$$
 where  $I_0 = h_0^3 / 12$  (5)

First-order Perturbation Equation

$$EI_{0} \frac{d^{4} w_{I}}{dx^{4}} - \rho h_{0} \omega_{0}^{2} w_{I}$$

$$= -\frac{Eh_{0}^{2}}{4} \left[ h_{I} \frac{d^{4} w_{0}}{dx^{4}} + \frac{2dh_{I}}{dx} \frac{d^{3} w_{0}}{dx^{3}} + \frac{d^{2}h_{I}}{dx^{2}} \frac{d^{2}w_{0}}{dx^{2}} \right]$$

$$+ \rho \left[ \omega_{0}^{2} h_{I} w_{0} + \omega_{I}^{2} h_{0} w_{0} \right]$$
(6)

Second-order Perturbation Equation

$$EI_{0} \frac{d^{4}w_{2}}{dx^{4}} - \rho h_{0} \omega_{0}^{2} w_{2}$$

$$= -\frac{Eh_{0}^{2}}{4} \left[ h_{I} \frac{d^{4}w_{I}}{dx^{4}} + \frac{2dh_{I}}{dx} \frac{d^{3}w_{I}}{dx^{3}} + \frac{d^{2}h_{I}}{dx^{2}} \frac{d^{2}w_{I}}{dx^{2}} \right]$$

$$- \frac{Eh_{0}^{2}}{4} \left[ h_{I}^{2} \frac{d^{4}w_{0}}{dx^{4}} + 2 \left( \frac{dh_{I}}{dx} \right)^{2} \frac{d^{3}w_{0}}{dx^{3}} + \left( \frac{d^{2}h_{I}}{dx^{2}} \right)^{2} \frac{d^{2}w_{0}}{dx^{2}} \right]$$

$$+ \rho \left[ \omega_{2}^{2} h_{0} w_{0} + \omega_{I}^{2} h_{I} w_{0} + \omega_{0}^{2} h_{I} w_{I} + \omega_{I}^{2} h_{0} w_{I} \right]$$
(7)

The zeroth-order perturbation equation (5) is identical to the differential equation describing free fundamental motion of a beam with height  $h_0$ . Thus  $\omega_0$  and  $w_0$  can be determined for a given set of support conditions. With  $\omega_0^2$  and  $w_0$  known, then  $\omega_1^2$  and  $w_1$  can be found from Eq. (6) and the process continued to determine  $\omega_2^2$  and  $w_2$  from Eq. (7). Assume now that the beam is simply supported with length  $\ell$ . The exact solution to Eq. (5) for the fundamental mode is well known. To find a general solution to the first- and second-order perturbation equations (6) and (7), the perturbated modal functions  $w_i(x)$  in Eq. (4) are represented as a sum of the eigenfunctions of the simply supported uniform beam

$$w_i(x) = \sum_{i=1}^J w_{ij} \sin (j\pi x/\ell)$$
 (8)

The expressions for  $w_0$  and  $w_i$  are then substituted into Eq. (6) and the resulting equation multiplied by one of the eigenfunctions and integrated over the length of the beam. One then obtains

$$\omega_I^2 = \frac{2h_0}{\rho \ell} \int_0^\ell \left\{ \frac{Eh_0^2}{4} \right\}$$

$$\times \left[ h_I \frac{\pi^4}{\ell^4} \sin \frac{\pi x}{\ell} - \frac{2\pi^3}{\ell^3} \frac{\mathrm{d}h_I}{\mathrm{d}x} \cos \frac{\pi x}{\ell} - \frac{\pi^2}{\ell^2} \frac{\mathrm{d}^2 h_I}{\mathrm{d}x^2} \sin \frac{\pi x}{\ell} \right]$$

$$-\rho\omega_0^2 h_1 \sin \frac{\pi x}{\ell} \right\} \sin \frac{\pi x}{\ell} dx \tag{9}$$

The relative amplitudes of the coefficients in Eq. (8) can also be determined as

$$\frac{w_{lk}}{w_{0l}} = \frac{-2h_0}{\rho_0 \ell \pi^4 k^4} \int_0^{\ell} \left\{ \frac{Eh_0^2}{4} \right.$$

$$\times \left[ h_l \frac{\pi^4}{\ell^4} \sin \frac{\pi x}{\ell} - \frac{2\pi^3}{\ell^3} \frac{\mathrm{d}h_l}{\mathrm{d}x} \cos \frac{\pi x}{\ell} - \frac{\pi^2}{\ell^2} \frac{\mathrm{d}^2 h_l}{\mathrm{d}x^2} \sin \frac{\pi x}{\ell} \right]$$

$$-\rho\omega_0^4 h_I \sin \frac{\pi_x}{\ell} \sin \frac{k\pi x}{\ell} dx \quad (k \neq I)$$
 (10)

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The development for the second-order correction is obtained from Eq. (7) in similar manner but the development is not included here

#### **B.** Free Vibration of Square Plates

The differential equation describing the free transverse motion of a square thin plate with variable thickness is

$$\nabla^{2}(D\nabla^{2}w) - (1-v)\left[\frac{\partial^{2}D}{\partial v^{2}}\frac{\partial^{2}w}{\partial x^{2}} - \frac{2\partial^{2}D}{\partial x\partial v}\frac{\partial^{2}w}{\partial x\partial v} + \frac{\partial^{2}D}{\partial x^{2}}\frac{\partial^{2}w}{\partial v^{2}}\right] - \omega^{2}\rho hw = 0$$
(11)

where  $\nu$  is Poisson's ratio and D(x,y) is the plate flexural rigidity. Again, the thickness variation is assumed to be only slightly different from some reference uniform plate as given in Eq. (2). The frequency and mode shape of the plate with variable thickness are given by Eqs. (3) and (4) which represent only slight variations of the fundamental frequency and mode shape from the reference uniform plate. Equations (2-4) are substituted into Eq. (11) and terms of the same power of  $\epsilon$  are equated to obtain the following set of differential equations

Zeroth-order Perturbation Equation

$$D^{I}h_{0}^{3}\nabla^{4}w_{0} - \rho\omega_{0}^{2}h_{0}w_{0} = 0$$
 (12)

First-order Perturbation Equation

$$D^{I}h_{0}^{3}\nabla^{A}w_{I} - \rho\omega_{0}^{2}h_{0}w_{I} = -D^{I}h_{0}^{2}\left\{3\nabla^{2}h_{I}\nabla^{2}w_{0} + 3h_{I}\nabla^{A}w_{0} + 6\frac{\partial h_{I}}{\partial x}\frac{\partial}{\partial x}\left(\nabla^{2}w_{0}\right) + 6\frac{\partial h_{I}}{\partial y}\frac{\partial}{\partial y}\left(\nabla^{2}w_{0}\right) - (I-v)\left[3\frac{\partial^{2}h_{I}}{\partial y^{2}}\frac{\partial^{2}w_{0}}{\partial x^{2}}\right] + 3\frac{\partial^{2}h_{I}}{\partial x^{2}}\frac{\partial^{2}w_{0}}{\partial y^{2}} - 6\frac{\partial^{2}h_{I}}{\partial x\partial y}\frac{\partial^{2}w_{0}}{\partial x\partial y}\right]\right\} + \rho\left\{\omega_{0}^{2}h_{I}w_{0} + \omega_{I}^{2}h_{0}w_{0}\right\}$$

$$(13)$$

Second-order Perturbation Equation

$$D^{I}h_{0}^{3}\nabla^{4}w_{2} - \rho_{0}\omega_{0}^{2}h_{0}w_{2} = -D^{I}\left\{3h_{0}^{2}\nabla^{2}h_{1}\nabla^{2}w_{1} + 3h_{0}\nabla^{2}h_{1}^{2}\nabla^{2}w_{0} + 3h_{0}^{2}h_{1}\nabla^{4}w_{1} + 3h_{0}h_{1}^{2}\nabla^{4}w_{0} + 6h_{0}^{2}\frac{\partial}{\partial x}\left(\nabla^{2}w_{1}\right)\right.$$

$$\left. + 6h_{0}\frac{\partial}{\partial x}h_{1}^{2}\frac{\partial}{\partial x}\left(\nabla^{2}w_{0}\right) + 6h_{0}\frac{\partial}{\partial y}h_{1}^{2}\frac{\partial}{\partial y}\left(\nabla^{2}w_{0}\right) - (I-v)\left[3h_{0}^{2}\frac{\partial^{2}}{\partial y^{2}}h_{1}\frac{\partial^{2}}{\partial x^{2}}w_{1} + 3h_{0}\frac{\partial^{2}}{\partial y^{2}}h_{1}^{2}\frac{\partial^{2}}{\partial x^{2}}w_{0} + 3h_{0}^{2}\frac{\partial^{2}}{\partial x^{2}}h_{1}\frac{\partial^{2}}{\partial y^{2}}w_{1}\right.$$

$$\left. + 3h_{0}\frac{\partial^{2}}{\partial x^{2}}h_{1}^{2}\frac{\partial^{2}}{\partial y^{2}}w_{0} - 6h_{0}^{2}\frac{\partial^{2}h_{1}}{\partial x\partial y}\frac{\partial^{2}w_{1}}{\partial x\partial y} - 6h_{0}\frac{\partial^{2}h_{1}}{\partial x\partial y}\frac{\partial^{2}w_{0}}{\partial x\partial y}\right]\right\} + \rho\{\omega_{1}^{2}h_{0}w_{1} + \omega_{1}^{2}h_{1}w_{0} + \omega_{2}^{2}h_{0}w_{0}\}$$

$$(14)$$

The fundamental mode shape of the plate with variable thickness is represented by a series of eigenfunctions of the reference uniform plate

$$w_{i}(x,y) = \sum_{m=1}^{M} \sum_{n=1}^{N} w_{i_{mn}} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{a}$$
 (15)

Expressions for  $w_0$  and  $w_1$  are substituted into Eq. (13), which is satisfied in the Fourier sense to obtain a first-order correction for frequency.

$$\omega_{I}^{2} = \frac{4}{\rho h_{0} a^{2}} \int_{0}^{a} \left\{ \left[ -3 \left( 1 + v \right) D^{T} h_{0}^{2} \left( \frac{\pi}{a} \right)^{2} \nabla h_{I} + 12 D^{T} h_{0}^{2} \left( \frac{\pi}{a} \right)^{2} h_{I} - \rho \omega_{0}^{2} h_{I} \right] \sin^{2} \frac{\pi x}{a} \sin^{2} \frac{\pi y}{a}$$

$$-12 D^{T} h_{0}^{2} \left( \frac{\pi}{a} \right)^{3} \left[ \frac{\partial h_{I}}{\partial x} \cos \frac{\pi x}{a} \sin \frac{\pi x}{a} \sin^{2} \frac{\pi y}{a} - \frac{\partial h_{I}}{\partial y} \sin^{2} \frac{\pi x}{a} \cos \frac{\pi y}{a} \sin \frac{\pi y}{a} \right]$$

$$+6 (1 - v) D^{T} h_{0}^{2} \left( \frac{\pi}{a} \right)^{2} \frac{\partial^{2} h_{I}}{\partial x \partial y} \cos \frac{\pi x}{a} \sin \frac{\pi x}{a} \cos \frac{\pi y}{a} \sin \frac{\pi y}{a} \right\} dx dy$$

$$(16)$$

The magnitudes of  $w_{I_{min}}$  in Eq. (15) are

$$\frac{w_{I_{mn}}}{w_{0_{II}}} = \frac{-4a^2}{(m^4 + 2m^2n^2 + n^4)\pi^4} \times \int_0^a \int_0^a \left\{ \left[ -3(I + v) \left( \frac{\pi}{a} \right)^2 \nabla^2 h_I + 12 \left( \frac{\pi}{a} \right)^4 h_I - \frac{\rho}{D^I h_0^2} \omega_0^2 h_I \right] \sin \frac{\pi x}{a} \sin \frac{\pi y}{a} \right\} \\
-12 \left( \frac{\pi}{a} \right)^3 \left[ \frac{\partial h_I}{\partial x} \cos \frac{\pi x}{a} \sin \frac{\pi y}{a} + \frac{\partial h_I}{\partial y} \sin -6(I - v) \left( \frac{\pi}{a} \right)^2 \frac{\partial^2 h_I}{\partial x \partial y} \cos \frac{\pi x}{a} \cos \frac{\pi y}{a} \right\} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{a} \, dy dx \tag{17}$$

The second-order correction to frequency and mode shape is obtained in a similar manner from Eq. (14), but is not presented in detail here.

Table 1 Fundamental frequency of a tapered beam and a plate tapered in one direction

	Bed	am	
Taper ratio, $\alpha$	Fundamental frequency $\omega$ ( $m\ell/EI_0$ ) $^{1/2}$		
	Perturbation	Ref. 2	% Difference
.5	7.11	7.12	0.2
.67	5.97	6.06	1.5
.75	5.36	5.34	0.3
.90	3.99	3.88	2.8

Taper ratio, α	Plate  Fundamental frequency $\omega^2( ho h_0 a^4/D_0)$			
	Perturbation	Ref. 3	% Difference	
.2	748.2	748.2	0.00	
.4	649.4	650.1	0.11	
.6	556.6	557.2	0.10	
.8	469.7	470.5	0.17	

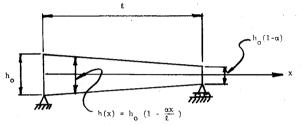


Fig. 1 Linearly tapered beam or cross section of a plate linearly tapered in the x direction.

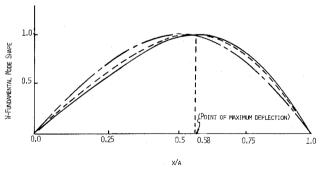


Fig. 2 Fundamental mode shape of a square plate linearly tapered in one direction (taper ratio  $\alpha = 0.8$ , y = a/2). Legend: (-) perturbation; (----) Ref. 3; (---) plate with uniform thickness.

# III. Examples

# A. Linearly Tapered Beam

The linearly tapered beam shown in Fig. 1 was selected as an example. The thickness variation can then be expressed as

$$\epsilon h_I(x) = -\alpha h_0 x/\ell; \ \epsilon = \alpha; \ h_I(x) = -h_0 x/\ell \tag{18}$$

The approximate fundamental frequency and mode shape are determined from Eqs. (9) and (10) and similar equations representing the second-order corrections. The change of fundamental frequency of the tapered beam is shown in Table 1 for four values of taper ratio and is compared with the exact frequency obtained from Ref. 2.

#### **B.** Linearly Tapered Plate

A simply supported square plate with linearly varying thickness in the x direction as shown in Fig. 1 was considered. The thickness of such a plate can be functionally represented as in Eq. (18). The approximate frequency for this particular

plate with variable thickness was calculated from Eq. (16) and a similar equation representing a second-order correction. The frequency change for various values of taper ratio,  $\alpha$ , is shown in Table 1 and compared with the frequency determined from Ref. 3. The fundamental mode shape as determined from Eq. (17) is shown in Fig. 2 and compared to the mode shape obtained from Ref. 3. The mode shape as determined from Eq. (17) is

$$w = [\sin(\pi x/a) + .1784 \sin(2\pi x/a) + .004 \sin(4\pi x/a)]$$

$$+.0005 \sin(6\pi x/a) \sin(\pi y/a)$$
 (19)

## References

<sup>1</sup>Chehil, D. S. and Dua, S. S., "Buckling of Rectangular Plates with General Variation in Thickness," *Journal of Applied Mechanics*, Vol. 40, No. 3, Sept. 1973, pp. 745-751.

<sup>2</sup>Conway, H. D. and Dubil, J. F., "Vibration Frequencies of Truncated-Cone and Wedge Beams," *Journal of Applied Mechanics*, Vol. 32, No. 4, Dec. 1965, pp. 932-934

32, No. 4, Dec. 1965, pp. 932-934.

<sup>3</sup> Appl, F. C. and Byers, N. F., "Fundamental Frequency of Simply Supported Rectangular Plates with Varying Thicknesses," *Journal of Applied Mechanics*, Vol. 32, No. 1, March 1965, pp. 163-168.

<sup>4</sup>J. D. Cole, *Perturbation Methods in Applied Mathematics*, Gin/Blaisdell Publishing Company, Waltham, Mass., 1968, pp. 105-111

# Two-Dimensional Radiative Equilibrium: A Simple Nongray Problem

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RECENT review<sup>1</sup> of the literature on two-dimensional radiative equilibrium reveals that all of the analyses use the gray approximation. The spectra of many substances, such as glass, carbon monoxide, and water vapor have windows or regions in which the absorption coefficient is zero or very small. The inability of the gray model to account for these windows is one of its major limitations. The present study of nongray radiative transfer in a two-dimensional

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